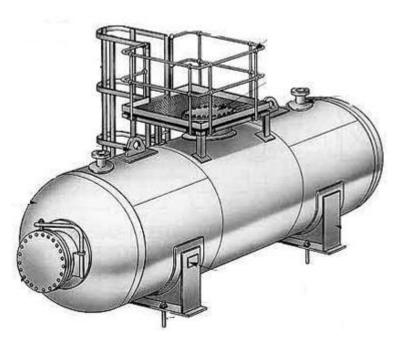
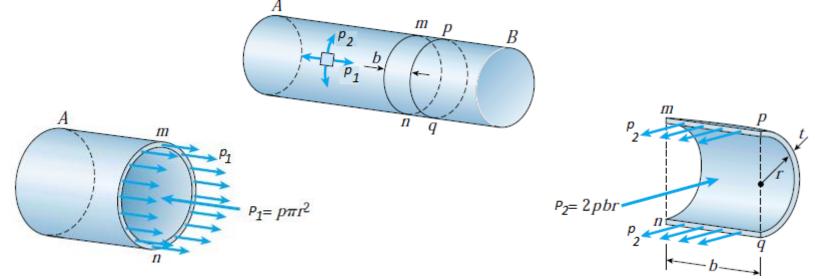
Thin Cylindrical Shells and Spheres



- Water supply main pipes, steam boilers, gas pipes and air vessels etc. are subjected to internal fluid pressures.
- Pressures are taken to be uniformly distributed over internal surfaces.
- Wall thickness of pressure vessels are so small compared to the large cross-sectional internal diameter.
- If wall thickness is equal to or less than 1/20 of the internal diameter the vessel ⇒ Thin walled.
- Otherwise \Rightarrow **Thick walled**.
- Lecture is concerned with **stresses arising** from uniform internal pressure acting on a cylinder or a sphere or cylinder with spherical ends
- The following is assumed:
 - 1) Radial stresses in cylinder walls are neglected
 - 2) There are no longitudinal stays in the cylinder
 - 3) Stresses are **uniformly distributed** through the wall.

Thin Cylindrical Circular Shell

Consider the effect of an internal pressure *p* due to a fluid enclosed in along cylinder closed at both ends.



- Pressure acting at the ends is transmitted to the walls producing longitudinal tensile stress p₁ in the walls.
- Internal pressure acting on the long sided of the cylinder give rise to circumferential stress p₂ called Hoop stresses.

a) Circumferential Stress or Hoop Stress

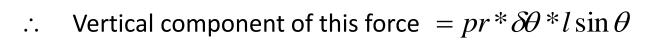
Consider an elementary portion of the cylinder wall of width δx subtending an angle $\delta \theta$ at the centre and θ with X – X.

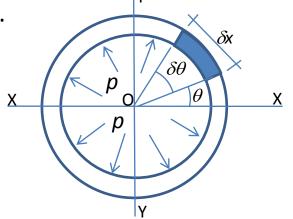
Let

- r Internal radius
- l overall length of cylinder.
- P'- Total internal pressure.

$$P' = p * \text{area of elementary portion}$$

= $p * \delta x * l$
= $p * r \delta \theta * l$ $\left(\because \frac{\delta x}{r} = \delta \theta \right)$





... Total vertical upward force on the semicircular cylindrical shell above X-X

$$= \int_{0}^{\pi} prl\sin\theta\,\delta\theta$$
$$= prl[-\cos\theta]_{0}^{\pi} = p*2r*l$$
$$= p*\text{horizontal projected area of the shell}$$

- Similarly, the vertical downward force on the semicircular portion of the cylindrical below X-X is also *p x 2r x l*.
- These two equal and opposite forces tend to burst the shell longitudinally at the plane X-X.
- This bursting force is balanced by resisting **Hoop stress** created.

$$\therefore \qquad p2rl = 2p_h tl$$
or
$$p_h = \frac{pr}{t} \qquad \dots (1)$$

b) Longitudinal Stress

Consider a shell of wall thickness *t* and radius *r* subjected to internal pressure *p*. Resultant force of the internal pressure at the end is $P = p * \pi r^2$

If p_l - Longitudinal resisting stress developed in the shell, then

$$P = p_l * 2\pi r * t$$

or $p * \pi r^2 = p_l * 2\pi r * t$
$$\therefore \qquad p_l = \frac{p * \pi r^2}{2\pi r * t} = \frac{pr}{2t} \qquad \dots (2)$$

Hoop stress p_h is thus twice the longitudinal stress. As such, of the two stress Hoop stress p_h is the maximum.

c) Maximum Shear Stress

- At any point on the circumference of the cylindrical shell there is set of two mutually perpendicular stresses p_h and p_l which are principal stresses and as such the planes in which these act are the principal planes.
- ... The maximum shear stress is

$$q_{\max} = \frac{p_h - p_l}{2}$$
$$= \frac{\frac{pr}{t} - \frac{pr}{2t}}{2} = \frac{pr}{4t} \qquad \dots (3)$$

Design of Thin Cylindrical Shells

If wall thickness of a thin cylindrical shell is desired to be determined so that it can withstand a given internal pressure p then we have to ensure that the maximum stress developed in the shell does not exceed the allowable tensile stress $[p_t]$ for the material of the shell.

Since the hoop stress is bigger, so we design the shell based on it.

But

$$p_h = \frac{pr}{t}$$
, where t - required shell thickness.

But p_h is not to exceed p_t

$$\therefore \quad \frac{pr}{t} \le p_t$$

or
$$t \ge \frac{pr}{[p_t]}$$

Strains in Thin Cylindrical Shell due to Internal Pressure

lf

If for

t – thin cylinder wall thickness

r – cylinder radius

p – internal fluid pressure

Hoop stress and longitudinal stress:

$$p_h = \frac{pr}{t}$$
 and $p_l = \frac{pr}{2t}$
the material that the shell is made of

Poisson's ratio
$$= \frac{1}{m}$$

and Modulus of elasticity = E

then circumferential or Hoop strain is :

$$e_h = \frac{1}{E} \left(p_h - \frac{1}{m} p_l \right)$$

Substituting values for p_h and p_l we have:

$$e_{h} = \frac{1}{E} \left(\frac{pr}{t} - \frac{pr}{2tm} \right)$$
$$= \frac{pr}{tE} \left(1 - \frac{1}{2m} \right) \qquad \dots (5)$$

Longitudinal strain

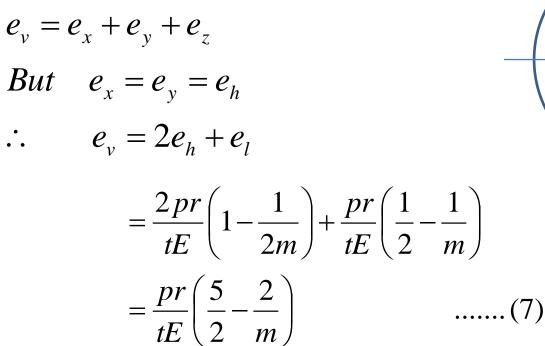
$$e_l = \frac{1}{E} \left(p_l - \frac{1}{m} p_h \right)$$

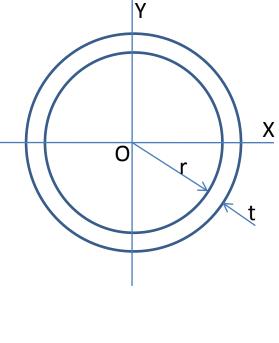
Substituting values for p_h and p_l we have:

$$e_{l} = \frac{1}{E} \left(\frac{pr}{2t} - \frac{pr}{tm} \right)$$
$$= \frac{pr}{tE} \left(\frac{1}{2} - \frac{1}{m} \right) \qquad \dots (6)$$

Strain along OX and OY is equal and is e_h whereas along axis of the cylinder it is e_l .

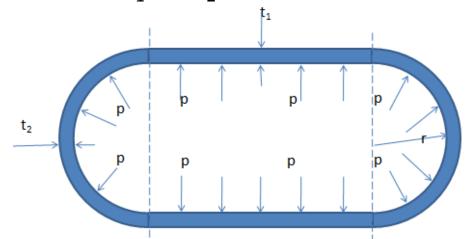






Thin Cylindrical Shells with Hemispherical Ends

Let r be the radius of the shell and respective wall thickness of the cylindrical and spherical ends be t_1 and t_2 with internal pressure p.



If **p** be internal pressure shell is subjected to, then the hoop strain:

For cylindrical portion is:

$$e_{h} = \frac{pr}{t_{1}E} \left(1 - \frac{1}{2m}\right) \qquad \dots (i) \qquad (\because (5))$$

For hemispherical ends is:

$$e = \frac{pr}{2t_2 E} \left(1 - \frac{1}{m} \right) \qquad \dots \text{(ii)}$$

So that there is no distortion at the junction of cylindrical and hemispherical portions, the hoop strains in the two have to be equal,

$$e_h = e$$
 eqns (i) and (ii)

Assuming that both the portions are made of the same material, then

$$\frac{pr}{t_1 E} \left(1 - \frac{1}{2m} \right) = \frac{pr}{2t_2 E} \left(1 - \frac{1}{m} \right)$$

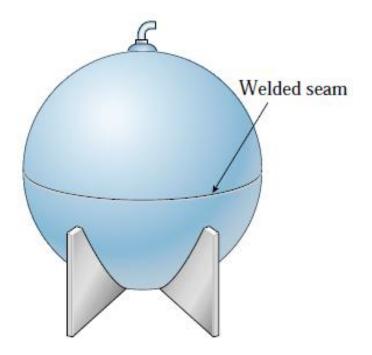
$$\frac{t_2}{t_1} = \frac{\left(1 - \frac{1}{m}\right)}{2\left(1 - \frac{1}{2m}\right)} = \frac{(m-1)}{(2m-1)} \qquad \dots (8)$$

or
$$t_2: t_1 :: (m-1): (2m-1)$$

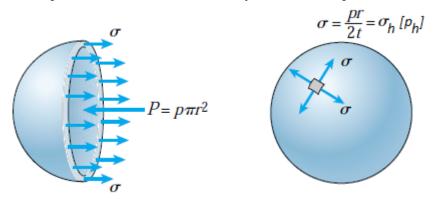
 t_2 shall always be less that t_1 regardless the value of m_2 , the hemispherical end is always thinner than cylindrical portion.

. .

Thin Spherical Shell



Consider a diametral plane through the shell, internal radius *r* and wall thickness *t* subjected to internal pressure *p*.



Resultant force of internal pressure normal to the diametral plane is

$$P = p * \pi r^2$$

For equilibrium, we equally have resisting force

$$P = p_h * 2\pi r * t$$

Therefore

$$p * \pi r^{2} = p_{h} * 2\pi r * t$$

or
$$p_{h} = \frac{p * \pi r^{2}}{2\pi r * t} = \frac{pr}{2t} \qquad \dots (9)$$

Since stress p_h is the same in all the direction X,Y and Z-axis, therefore strain too in all the three planes are same. The diameter of the shell in all diametral plane is strained by an amount:

$$e = \frac{1}{E} \left(p_h - \frac{1}{m} p_h \right) = \frac{p_h}{E} \left(1 - \frac{1}{m} \right)$$
$$= \frac{pr}{2tE} \left(1 - \frac{1}{m} \right) \qquad \dots (10) \qquad \left(\because p_h = \frac{pr}{2t} \right)$$

Since stress p_h is the same in all the direction X,Y and Z-axis, therefore strain too in all the three planes are same.

Therefore, volumetric strain

$$e_{v} = e_{x} + e_{y} + e_{z} = e + e + e = 3e$$
$$\frac{3p_{h}}{E} \left(1 - \frac{1}{m}\right) = \frac{3pr}{2tE} \left(1 - \frac{1}{m}\right) \quad \dots (11)$$

End of Lecture