

THE UNIVERSITY OF ZAMBIA
SCHOOL OF ENGINEERING
DEPARTMENT OF MECHANICAL ENGINEERING

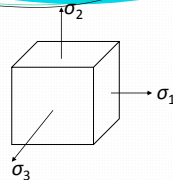
MEC 3352 – STRENGTH OF MATERIALS II

Principal Strains in Three Dimensions

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The 3D Principal Strains

From the 2D case, it can be shown that the principal strains in 3D σ_1 , σ_2 and σ_3 are:



$$\epsilon_1 = \sigma_1/E - \nu\sigma_2/E - \nu\sigma_3/E \quad (1)$$

$$\epsilon_2 = \sigma_2/E - \nu\sigma_3/E - \nu\sigma_1/E \quad (2)$$

$$\epsilon_3 = \sigma_3/E - \nu\sigma_1/E - \nu\sigma_2/E \quad (3)$$

Principal Stresses Determined from Principal Strains

(a) 3D Stress System:

We re-writing eqs. (1), (2) and (3):

$$E\epsilon_1 = \sigma_1 - \nu\sigma_2 - \nu\sigma_3 \quad (4)$$

$$E\epsilon_2 = \sigma_2 - \nu\sigma_3 - \nu\sigma_1 \quad (5)$$

$$E\epsilon_3 = \sigma_3 - \nu\sigma_1 - \nu\sigma_2 \quad (6)$$

• Subtracting (5) from (4):

$$E(\epsilon_1 - \epsilon_2) = (\sigma_1 - \sigma_2)(1 + \nu) \quad (7)$$

From (1) and (3), we eliminate σ_3 :

$$E(\varepsilon_1 + v\varepsilon_3) = \sigma_1(1 - v^2) - \sigma_2(1 + v)v \quad (8)$$

Multiplying (7) by v and subtracting from (8):

$$\begin{aligned} E[(1-v)\varepsilon_1 + v(\varepsilon_2 + \varepsilon_3)] \\ &= \sigma_1(1 - v^2) - \sigma_2(1 + v)v - (\sigma_1 - \sigma_2)(1 + v)v \\ &= \sigma_1(1 - v - 2v^2) \\ &= \sigma_1(1 + v)(1 - 2v) \end{aligned}$$

Rearranging:

$$\sigma_1 = \frac{E[(1-v)\varepsilon_1 + v(\varepsilon_2 + \varepsilon_3)]}{(1+v)(1-2v)} \quad (9)$$

Similarly

$$\sigma_2 = \frac{E[(1-v)\varepsilon_2 + v(\varepsilon_3 + \varepsilon_1)]}{(1+v)(1-2v)} \quad (10)$$

and

$$\sigma_3 = \frac{E[(1-v)\varepsilon_3 + v(\varepsilon_1 + \varepsilon_2)]}{(1+v)(1-2v)} \quad (11)$$

(b) 2D stress system.

If $\sigma_3 = 0$, (4), (5) and (6) reduce to:

$$E\varepsilon_1 = \sigma_1 - v\sigma_2 \quad (12)$$

$$E\varepsilon_2 = \sigma_2 - v\sigma_1 \quad (13)$$

Solving (12) and (13) for σ_1 and σ_2 gives

$$\sigma_1 = \frac{E(\varepsilon_1 + v\varepsilon_2)}{(1 - v^2)} \quad (14)$$

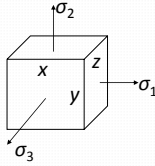
and

$$\sigma_2 = \frac{E(\varepsilon_2 + v\varepsilon_1)}{(1 - v^2)} \quad (15)$$

Volumetric Strain

Consider a block with sides x , y and z acted upon by principal stresses σ_1 , σ_2 and σ_3 .

Let corresponding linear strains be ϵ_1 , ϵ_2 and ϵ_3 .



Corresponding resultant dimensions are:

$$x + \epsilon_1 x, y + \epsilon_2 y \text{ and } z + \epsilon_3 z$$

or $x(1 + \epsilon_1)$, $y(1 + \epsilon_2)$ and $z(1 + \epsilon_3)$

Volumetric strain, is given as: $\epsilon_v = \frac{\text{Change in volume}}{\text{Original volume}}$

$$\epsilon_v = \frac{x(1 + \epsilon_1) \cdot y(1 + \epsilon_2) \cdot z(1 + \epsilon_3) - xyz}{xyz}$$

$$= (1 + \epsilon_1) \cdot (1 + \epsilon_2) \cdot (1 + \epsilon_3) - 1$$

Expanding and neglecting second order terms – since strains are small:

$$\epsilon_v = \epsilon_1 + \epsilon_2 + \epsilon_3 \quad (16)$$

Expressing the strains in terms of principal stresses (using Eqs. (1), (2) and (3)):

$$\text{Volumetric strain, } \epsilon_v = \frac{(\sigma_1 + \sigma_2 + \sigma_3)(1 - 2\nu)}{E} \quad (17)$$

Strain Energy

Strain energy, U = work done by a system of stresses in straining a material.

Consider a cube acted upon by a system of principal stresses σ_1 , σ_2 and σ_3 . For the corresponding strains of ϵ_1 , ϵ_2 and ϵ_3 ,

$$U = \sum \frac{1}{2} \sigma \epsilon$$

The work done (if the stresses are gradually applied).

$$\begin{aligned} U &= \frac{1}{2} \sigma_1 \epsilon_1 + \frac{1}{2} \sigma_2 \epsilon_2 + \frac{1}{2} \sigma_3 \epsilon_3 \\ &= [\sigma_1(\sigma_1 - \nu \sigma_2 - \nu \sigma_3) + \sigma_2(\sigma_2 - \nu \sigma_3 - \nu \sigma_1) \\ &\quad + \sigma_3(\sigma_3 - \nu \sigma_1 - \nu \sigma_2)] \quad \text{from (1), (2) and (3).} \end{aligned}$$

$$U = \left(\frac{1}{2E} \right) \left[\sigma_1^2 + \sigma_2^2 + \sigma_3^2 - 2\nu(\sigma_1\sigma_2 + \sigma_2\sigma_3 + \sigma_3\sigma_1) \right] \quad (18)$$

per unit volume.

For a 2D stress system, $\sigma_3 = 0$

$$U = \left(\frac{1}{2E} \right) \left[\sigma_1^2 + \sigma_2^2 - 2\nu(\sigma_1\sigma_2) \right] \quad (19)$$

per unit volume.

Shear Strain Energy

Writing

$$\sigma_1 = \frac{1}{3}(\sigma_1 + \sigma_2 + \sigma_3) + \frac{1}{3}(\sigma_1 - \sigma_2) + \frac{1}{3}(\sigma_1 - \sigma_3) \quad (20)$$

$$\sigma_2 = \frac{1}{3}(\sigma_1 + \sigma_2 + \sigma_3) + \frac{1}{3}(\sigma_2 - \sigma_3) + \frac{1}{3}(\sigma_2 - \sigma_1) \quad (21)$$

$$\sigma_3 = \frac{1}{3}(\sigma_1 + \sigma_2 + \sigma_3) + \frac{1}{3}(\sigma_3 - \sigma_1) + \frac{1}{3}(\sigma_3 - \sigma_2) \quad (22)$$

Then, under the action of the mean stress there, WILL BE volumetric strain with NO distortion of shape (i.e. no shear stress anywhere).

\therefore The strain energy under this mean stress acting in each direction is obtained from the general formula, Eq. (18), and may be called the *volumetric strain energy*,

$$U = \left(\frac{3}{2E} \right) \left[\frac{\sigma_1 + \sigma_2 + \sigma_3}{3} \right]^2 \cdot (1 - 2\nu)$$

Giving

$$U = \left(\frac{1}{6E} \right) \left[\sigma_1 + \sigma_2 + \sigma_3 \right]^2 \cdot (1 - 2\nu) \quad (23)$$

The other terms in the rearrangement of σ_1 , σ_2 and σ_3 are proportional to the maximum shear stress values in the three planes, and will cause a distortion of the shape.

Let U_s = Shear strain energy = Total strain energy – Volumetric strain energy

$$\begin{aligned}
 U_s &= \left(\frac{1}{2E} \right) \left[\sigma_1^2 + \sigma_2^2 + \sigma_3^2 - 2\nu(\sigma_1\sigma_2 + \sigma_2\sigma_3 + \sigma_3\sigma_1) \right] - \\
 &\quad \left(\frac{1}{6E} \right) \left[(\sigma_1 + \sigma_2 + \sigma_3)^2 \cdot (1 - 2\nu) \right] \\
 &= \left(\frac{1}{6E} \right) \left[(\sigma_1^2 + \sigma_2^2 + \sigma_3^2)(3 - 1 + 2\nu) - (\sigma_1\sigma_2 + \sigma_2\sigma_3 + \sigma_3\sigma_1)(6\nu + 2 - 4\nu) \right] \\
 &= \left(\frac{1 + \nu}{6E} \right) \left[2(\sigma_1^2 + \sigma_2^2 + \sigma_3^2) - 2(\sigma_1\sigma_2 + \sigma_2\sigma_3 + \sigma_3\sigma_1) \right] \\
 U_s &= \left(\frac{1 + \nu}{12G} \right) \left[(\sigma_1 - \sigma_2)^2 + (\sigma_2 - \sigma_3)^2 + (\sigma_3 - \sigma_1)^2 \right] \quad (24)
 \end{aligned}$$

since $E = 2G(1 + \nu)$.

The quantities in brackets are each twice the maximum shear stress in their respective planes.

In pure shear system (stress τ), the principal stresses are $\pm\tau, 0$ (review maximum shear stresses).

$$\therefore U_s = \left(\frac{1 + \nu}{12G} \right) \left[(2\tau)^2 + (-\tau)^2 + (-\tau)^2 \right]$$

(Since $\sigma_1 = \tau$, $\sigma_2 = -\tau$ when $\sigma_3 = 0$)

$$U_s = \frac{\tau^2}{2G} \quad (25)$$

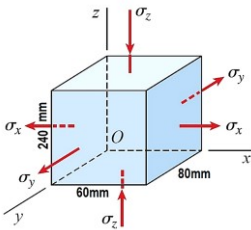
(Compare with strain energy in direct shear stress)

Examples

Example 001

A block of steel measuring 240 mm x 16 mm x 25 mm, is subjected to a tensile force of 40kN in the direction of its length. Given that the modulus of elasticity (E) and the Poisson's ratio (ν) are 200kN/mm² and 0.33 respectively. Compute the change in the volume of the block.

Examples



Example 002

A rectangular block measuring 240 mm x 80 mm x 60 mm is loaded as shown in the diagram, and $F_x = 80\text{kN}$, $F_y = 100\text{kN}$, $F_z = 40\text{kN}$. Compute the change in volume, the bulk modulus and modulus of rigidity. Take $E = 205\text{kN/mm}^2$ and $\nu = 0.36$.

ME 3352: Strength of Materials II

Enjoy MEC 3352.

Grazie
