

THE UNIVERISTY OF ZAMBIA

School of Engineering

Department of Civil and Environmental

Engineering

CEE 3111 - CIVIL ENGINEERING MATERIALS AND CONSTRUCTION PRACTICES

2023 ACADEMIC YEAR SEMESTER 1





TOPIC 6



Bituminous Materials

Part 2





3 Asphalt Concrete

Asphalt concrete, or hot-mix asphalt (HMA), is produced by mixing asphalt binder and aggregates at high temperatures and compacting them on the road.

The objective of the asphalt concrete mix design process is to provide resistance of the pavement to:

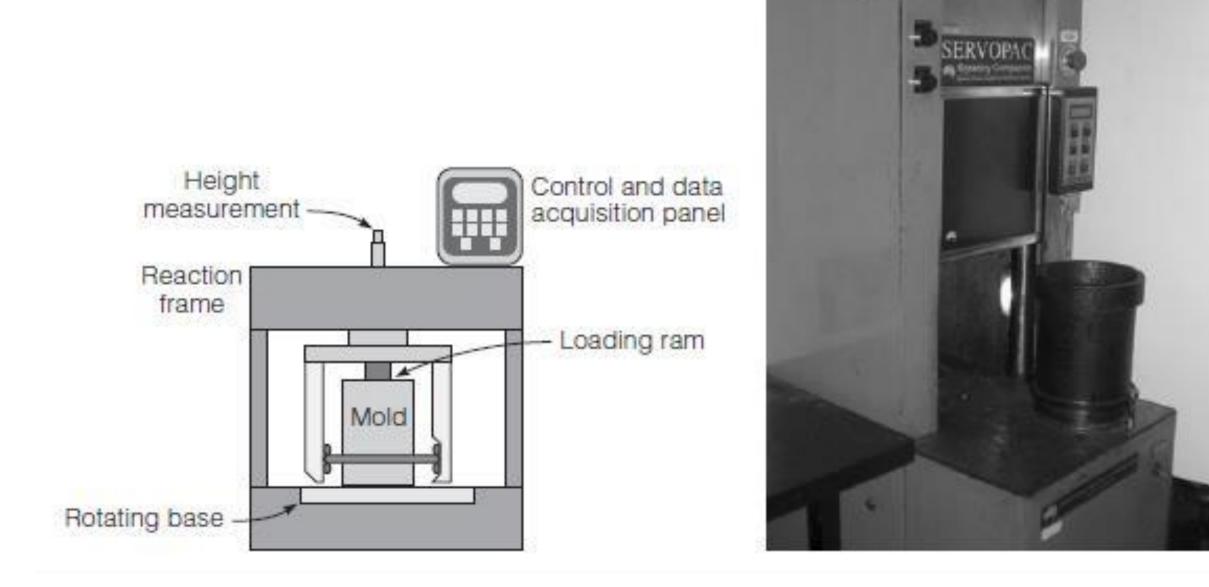
- Permanent deformation under traffic stresses, especially at high temperatures
- Fatigue cracking under repeated loadings
- Low-temperature contraction-induced thermal cracking
- Hardening or aging during production in the mixing plant and in service
- moisture-induced damage & skid resistance
- Workability, to reduce the effort needed during mixing, placing and compaction

Typical design asphalt contents range from 4% to 7% by weight of total mix





- Asphalt concrete design methods:
 - ✓ Superpave Mix design Superior Performing Asphalt Pavements (Uses Superpave) gyratory compactor)
 - ✓ <u>Marshall (ASTM D1559)</u> was commonly used (before superpave mix design) method was developed) for its simplicity and ability to use field control (Uses Marshall hammer)
 - ✓ Hveem (ASTM D1560)
- Both Marshall and Hveem approaches are empirical, based on past observations.
- The main difference between Marshall and Hveem techniques is how they determine the optimal asphalt content.
- Their empirical character makes them unsuitable for modified binders, big aggregates, and greater traffic loads.
- The Superpave design system is performance based and is more realistic than the Marshall and Hveem methods.



Superpave gyratory compactor



Marshall Compactor

- The basic steps required for performing Marshall mix design can be summarized into two:
 - Selection of Aggregates and
 - ii. Determination of Optimum Asphalt Content
- Asphalt Institute breaks down these steps into 6:
 - 1. Aggregate evaluation
 - 2. Asphalt cement evaluation
 - 3. Specimen preparation
 - 4. Marshall stability and flow measurement
 - 5. Density and voids analysis
 - 6. Design asphalt content determination

1. Aggregate Evaluation

- ✓ Durability & gradation ranges.
- ✓ soundness,
- ✓ presence of harmful elements,
- ✓ polishing, shape, & texture



2. Asphalt cement evaluation

Selection of asphalt cement grade depends on projected temperature range and traffic conditions.

3. Specimen preparation

The full Marshall mix-design procedure requires 18 specimens 101.6 mm in diameter

and 63.5 mm high.

The stability and flow are measured for 15 specimens.

In addition, 3 specimens are used to determine the theoretical maximum specific gravity G_{mm}



4. Marshall Stability and Flow Measurement

- Marshall stability maximum load asphalt concrete can carry when tested in the Marshall Apparatus
- Marshall flow deformation of the specimen when the load starts to decrease
- Stability is reported in newtons and flow is reported in units of 0.25 mm of deformation

5. Density and Voids Analysis

Involves determination of VTM, VMA, and VFA

6. Design Asphalt Content Determination

Traditionally, test results and calculations are tabulated and graphed to determine optimum asphalt content

Asphalt Institute Criteria for Marshall Mix Design (The Asphalt Institute, 2001)							
	Traffic Level						
	Light		Medium		Heavy		
Compaction (Blows)		35	50		50 75		75
	Minimum	Maximum	Minimum	Maximum	Minimum	Maximum	
Stability, kN (lb)	3.34 (750)	_	5.34 (1,200)	_	8.01 (1,800)		
Flow, 0.25 mm (0.01 in.)	8	18	8	16	8	14	
Air Voids, %	3	5	3	5	3	5	
VMA, %	Use	Criteria in T	able for min	imum percer	nt of VMA		
VFA, %	70	80	65	78	65	75	

Minimum Percent Voids in Mineral Aggregate (VMA) (The Asphalt Institute, 2001)

	Minimum VMA, Percent			
Nominal Maximum		esign Air Void:	s	
Particle Size ¹	3.0	4.0	5.0	
2.36 mm (No. 8)	19.0	20.0	21.0	
4.75 mm (No. 4)	16.0	17.0	18.0	
9.5 mm (3/8 in.)	14.0	15.0	16.0	
12.5 mm (1/2 in.)	13.0	14.0	15.0	
19.0 mm (3/4 in.)	12.0	13.0	14.0	
25.0 mm (1.0 in.)	11.0	12.0	13.0	

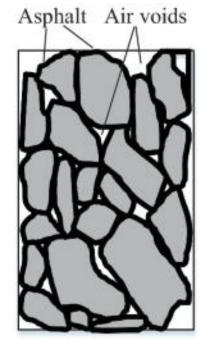
Density and Void Analysis

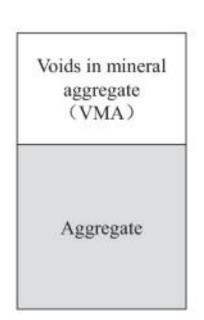
- Mix design and construction control require density and voids study of compacted asphalt mixtures, regardless of mix design method.
- Weights are utilized instead of volumes because volumes are difficult to measure; specific gravity is used to convert weight to volume.
- Three important parameters commonly used are:
 - ✓ Voids in Total Mix) (VTM) % of air voids between the coated aggregates compaction. Usually between 2-4%. Affects durability of asphalt pavement
 - 1. The lower the air voids, the less permeable.
 - 2. Water and air can enter the mix through too many air gaps.
 - 3. Low air voids can cause flashing or bleeding, as excess binder squeezes to the surface.

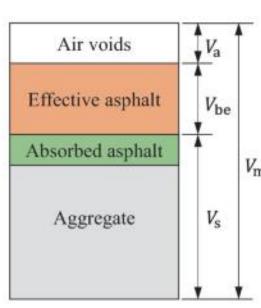
- ✓ Voids Filled with Asphalt (VFA)
- ✓ Voids in the Mineral Aggregate (VMA)
- Regardless of the compaction method, the samples are first used to determine the bulk specific gravity of the mix

$$G_{mb} = \frac{A}{B - C}$$









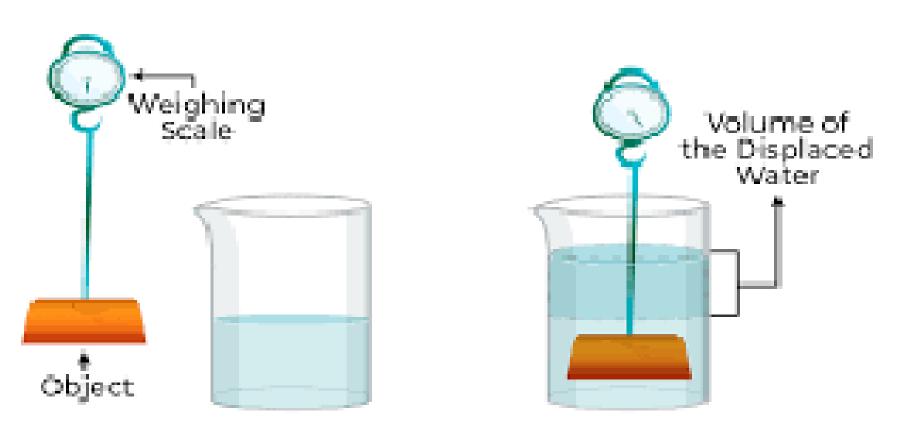
Where:

- G_{mh} = Bulk Specific Gravity of the Mix
- A = Dry Weight of Sample
- B = Saturated Surface Dry Weight of Sample
- C = Submerged Weight

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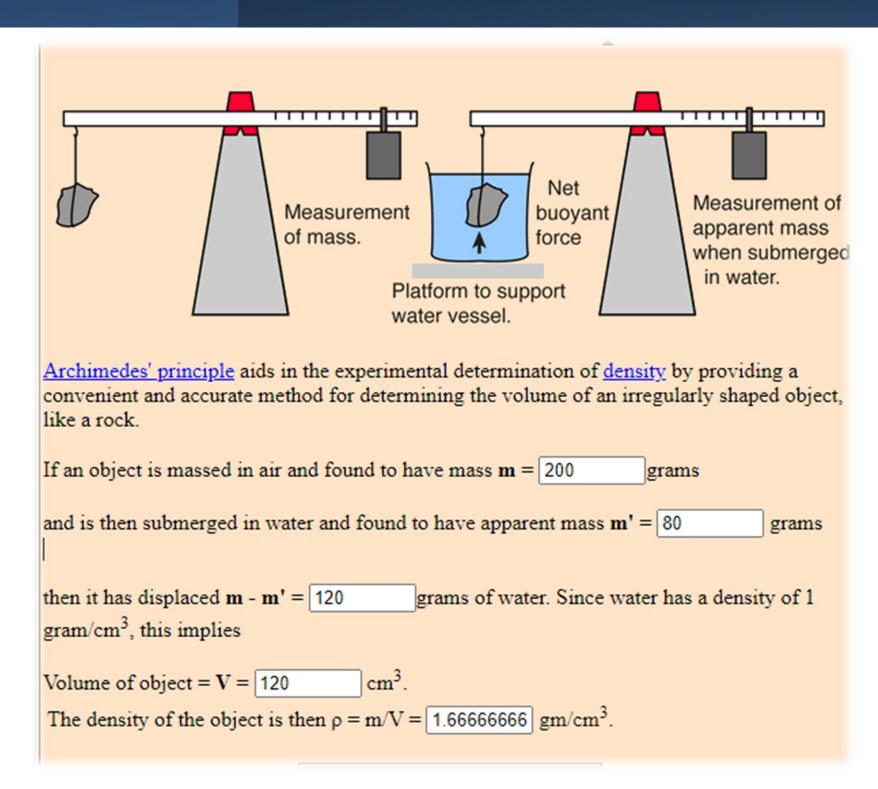
Asphalt Concrete Mix Design

Archimedes' principle



Upward Bouyant Force = Weight of the water displaced

= Volume of water displaced = Volume of object



The theoretical maximum specific gravity – Sp. Gr. excluding air voids

$$G_{mm} = \frac{A}{A + D - E}$$

Where:

- G_{mm} = Theoretical maximum specific gravity of the asphalt concrete
- A = dry weight of loose material
- D= calibration weight of the submerged vacuum bowl
- E= submerged weight of the vacuum bowl with the sample
- Once G_{mm} for sample at one asphalt content is determined, it can be used to estimate G_{mm} at other asphalt contents by first computing the effective specific gravity



$$Air\ Voids\ (percent) = \left(\frac{G_{mm} - G_{mb}}{G_{mm}}\right) \times 100$$

• Maximum theoretical specific gravity of uncompacted Samples: $G_{mm} = \frac{100}{\left(\frac{P_S}{G_{Se}} + \frac{P_b}{G_b}\right)}$ $G_{se} = \frac{100}{\left(\frac{100}{G_{mm}} - \frac{P_b}{G_b}\right)}$

$$G_{mm} = \frac{100}{\left(\frac{P_S}{G_{Se}} + \frac{P_b}{G_b}\right)}$$

$$G_{se} = \frac{P_s}{\left(\frac{100}{G_{mm}} - \frac{P_b}{G_b}\right)}$$



- Where:
- G_{mm} = Theoretical maximum specific gravity of the asphalt concrete
- $P_s = \%$ weight of aggregate
- $P_b = \%$ weight of the asphalt cement
- G_{se} = effective specific gravity of the aggregate coated with asphalt (determined for only one asphalt content, it is constant for all asphalt contents)
- The Marshal method uses this method for determining G_{mm} at each of the asphalt contents.
- The Superpave method requires directly measuring G_{mm} at each asphalt content evaluated during the mix design process

Density and Void Analysis

1. Voids in Total Mix

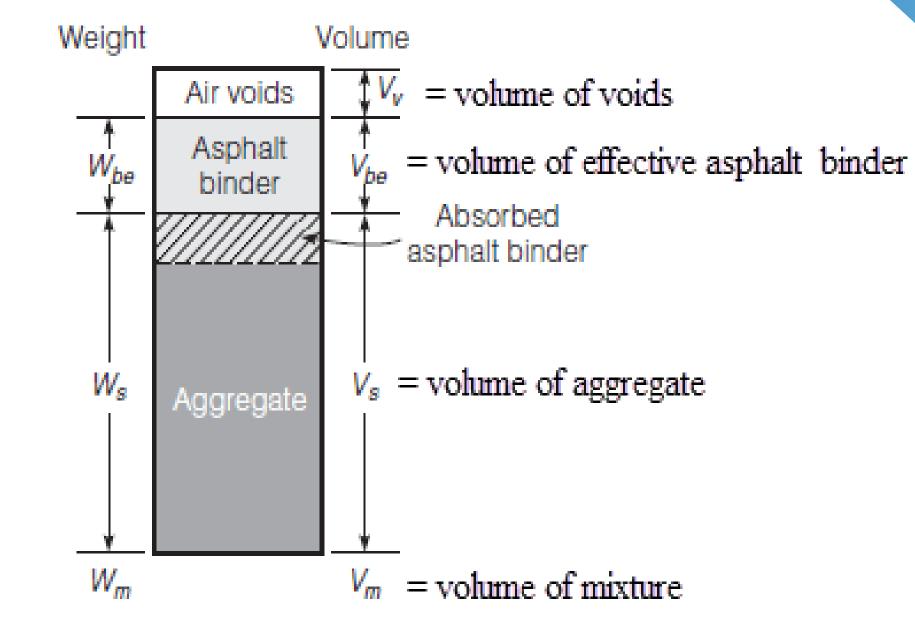
$$VTM = \frac{V_v}{V_m} \times 100 = 100(1 - \frac{G_{mb}}{G_{mm}})$$

2. Voids Filled with Asphalt

VFA =
$$\frac{V_{be}}{V_{be} + V_v} \times 100 = 100 \left(\frac{VMA - VTM}{VMA} \right)$$

3. Voids in Mineral Aggregate

$$VMA = \frac{V_V + V_{be}}{V_m} \times 100 = 100(1 - \frac{P_S}{G_{Sb}})$$



- G_{sh} = Specific gravity of the aggregate
- The effective asphalt is the total asphalt minus the absorbed asphalt

Density and Void Analysis – Example 1

 A compacted asphalt concrete specimen contains 5% asphalt binder (Sp. Gr. 1.023) by weight of total mix, and aggregate with a specific gravity of 2.755. The bulk density of the specimen is 2.441 Mg/m^3 . Ignoring absorption, compute VTM, VMA, and VFA.

Solution:

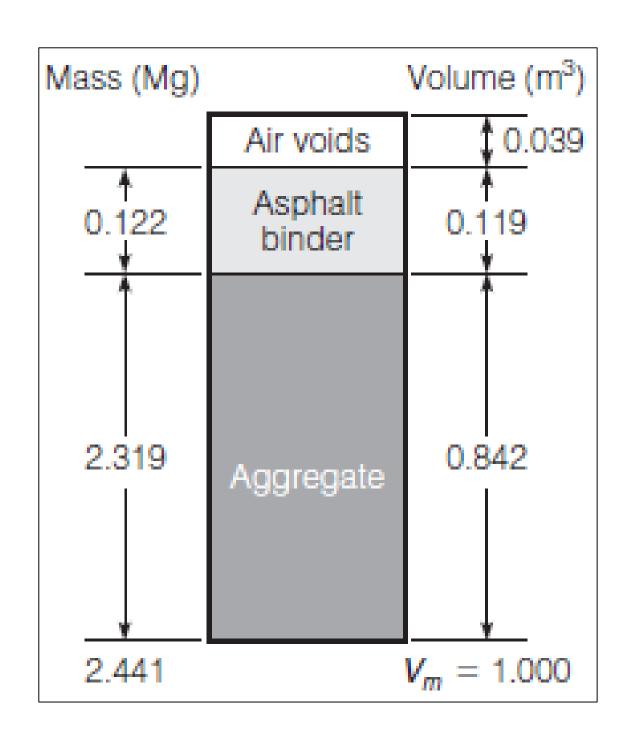
Referring to the figure:

Assume $Vm=1 m^3$

Determine mass of mix and components:

Total mass = 1 * 2.441 = 2.441 Mg

 \triangleright Mass of asphalt binder = 0.05 * 2.441 = 0.122 Mg Mass of aggregate = 0.95 * 2.441 = 2.319 Mg



Density and Void Analysis – Example 1

Determine volume of components:

$$V_b = \frac{0.122}{1.023} = 0.119m^3$$

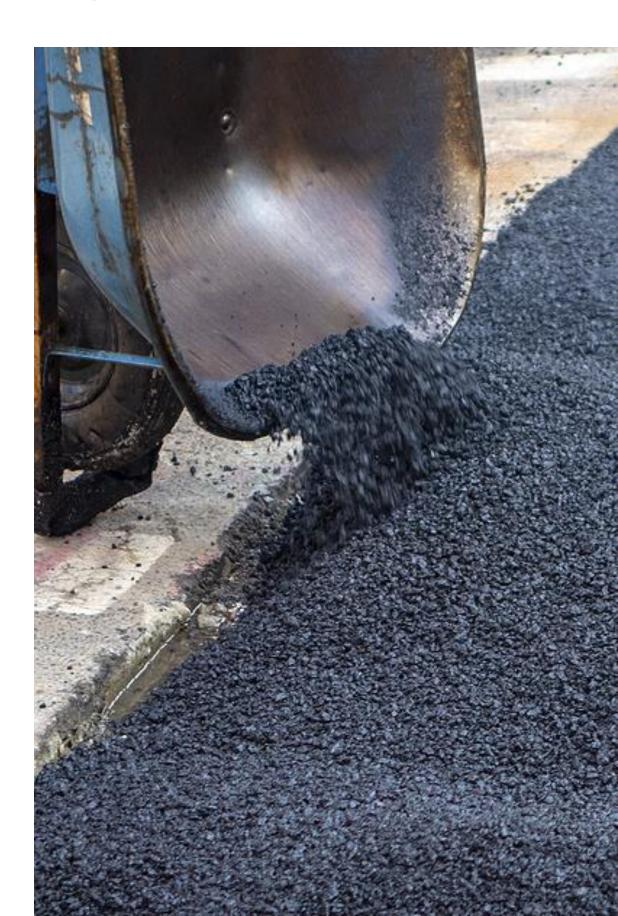
Since the problem statement specified no absorption, $V_{be} = V_b$ Determine $V_S = \text{Volume of Stone (Aggregate)}$

$$V_s = \frac{2.319}{2.755} = 0.842m^3$$

Determine V_{v} = Volume of Voids

$$V_S = V_m - V_S - V_b$$

 $V_S = 1 - 0.842 - 0.199 = 0.039m^3$



Density and Void Analysis – Example 1

Volumetric calculations:

$$VTM = \frac{V_v}{V_m} \times 100 = \frac{0.039}{1.00} \times 100 = 3.9\%$$

$$VMA = \frac{V_v + V_{be}}{V_m} \times 100 = \frac{0.039 + 0.119}{1.00} \times 100 = 15.8\%$$

$$VFA = \frac{V_{be}}{V_{be} + V_{v}} \times 100 = \frac{0.119}{0.119 + 0.039} \times 100 = 75\%$$

Volumetric Relationships used for asphalt mix design analysis – Example 2

An asphalt concrete specimen has the following properties:

- 1. Asphalt content = 5.9% by total weight of mix
- 2. Bulk specific gravity of the mix, Gmb = 2.457
- 3. Theoretical maximum specific gravity, Gmm = 2.598
- 4. Bulk specific gravity of aggregate, Gsb = 2.692 Calculate the percents VTM, VMA, and VFA.

Solution

$$VTM = 100 \left(1 - \frac{G_{mb}}{G_{mm}} \right) = 100 \left(1 - \frac{2.457}{2.598} \right) = 5.4\%$$

$$VMA = \left(100 - G_{mb} \frac{P_s}{G_{sb}} \right) = \left(100 - 2.457 \frac{100 - 5.9}{2.692} \right) = 14.1\%$$

$$VFA = 100 \left(\frac{VMA - VTM}{VMA} \right) = 100 \left(\frac{14.1 - 5.9}{14.1} \right) = 61.7\%$$

The Marshall method was used to design an asphalt concrete mixture. A PG 64-22 asphalt cement with a specific gravity G_b of 1.031 was used. The mixture contains a 9.5 mm nominal maximum particle size aggregate with a bulk specific gravity G_{sb} of 2.696. The theoretical maximum specific gravity G_{mm} of the mix at asphalt content of 5.0% is 2.470. Trial mixes were made with average results as shown in the following table:

Asphalt Content (P _b) (% by Weight of Mix)	Bulk Specific Gravity (G _{mb})	Corrected Stability (kN)	Flow (0.25 mm)
4.0	2.360	6.3	9
4.5	2.378	6.7	10
5.0	2.395	5.4	12
5.5	2.405	5.1	15
6.0	2.415	4.7	22

- Determine the design asphalt content using the Asphalt Institute design criteria for medium traffic.
- Assume a design air void content of 4%.

• Step 1: Compute G_{se} at 5% asphalt binder content and $G_{mm} = 2.470$

$$G_{se} = \frac{P_s}{(\frac{100}{G_{mm}} - \frac{P_b}{G_b})} = \frac{100 - 5}{\frac{100}{2.470} - \frac{5}{1.031}} = 2.666$$

• Step 2: Use G_{se} at 5% asphalt binder to calculate G_{mm} for the other asphalt contents At 4.0 % asphalt binder:

$$G_{se} = \frac{100}{\left(\frac{P_s}{G_{se}} - \frac{P_b}{G_b}\right)} = \frac{100}{\frac{100 - 4.0}{2.666} + \frac{4.0}{1.031}} = 2.51$$

Step 3: Compute VTM At 4.0 % asphalt binder:

$$VTM = 100 \left(1 - \frac{G_{mb}}{G_{mm}} \right) = 100 \left(1 - \frac{2.360}{2.51} \right) = 5.86\%$$

Step 4: Compute VMA At 4.0 % asphalt binder:

$$VMA = \left(100 - G_{mb} \frac{P_S}{G_{Sb}}\right) = \left(100 - 2.360 \frac{100 - 4}{2.696}\right) = 16\%$$

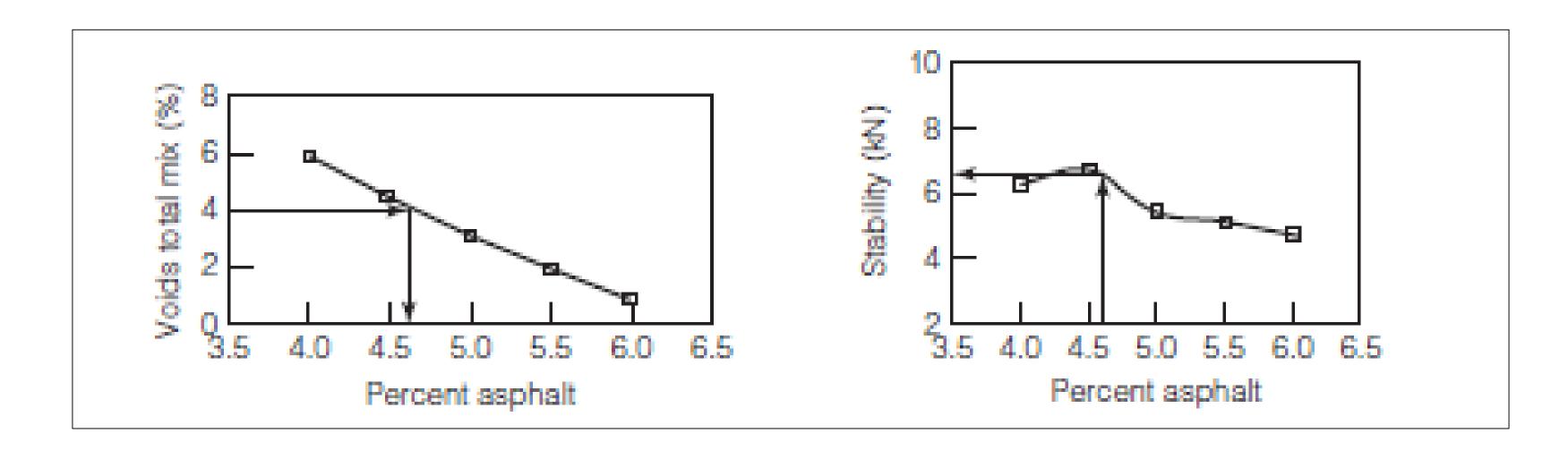
Step 5: Compute VFA At 4.0 % asphalt binder:

$$VFA = 100 \left(\frac{VMA - VTM}{VMA} \right) = 100 \left(\frac{16.0 - 5.86}{16.0} \right) = 63.3\%$$

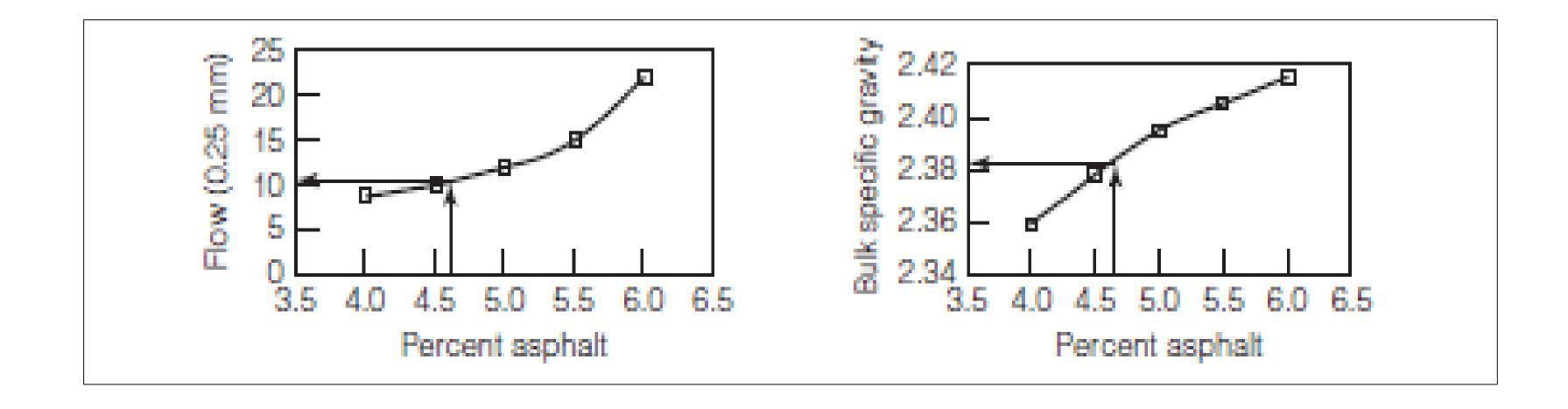


P _b (%)	G_{mb}	Corrected Stability (kN)	Flow, (0.25 mm)	$G_{\rm mm}$	G_{so}	VTM (%)	VMA (%)	VFA (%)
4.0	2.360	6.3	9	2.507		5.9	16.0	63.3
4.5	2.378	6.7	10	2.488		4.4	15.8	71.9
5.0	2.395	5.4	12	2.470	2.666	3.0	15.6	80.5
5.5	2.405	5.1	15	2.452		1.9	15.7	87.8
6.0	2.415	4.7	22	2.434		8.0	15.8	95.0

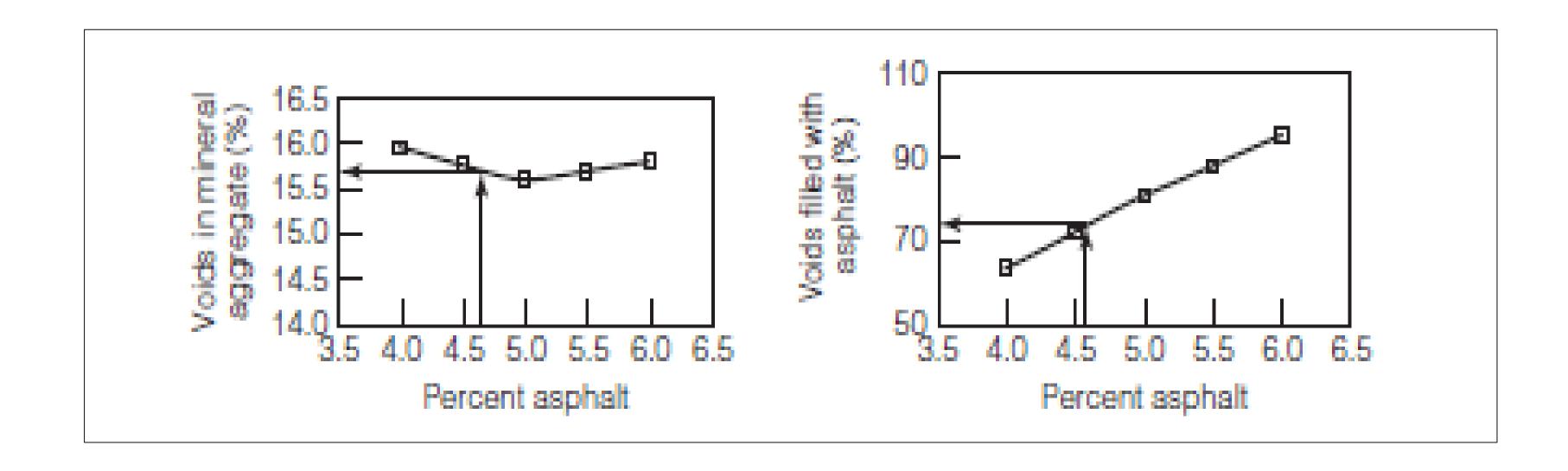
Step 6: Plot stability, flow and volumetric parameters versus asphalt content



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Step 6: Plot stability, flow and volumetric parameters versus asphalt content



Step 7: Determine the asphalt content at VTM=4% and the corresponding parameters. Compare with criteria:

At VTM = 4%, Find Corresponding Parameters from Graphs					
Parameter	From Graph	Criteria			
% of Asphalt binder at VTM = 4%	4.6				
Stability in N at 4.6 % Binder	6600	5400 (min)			
Flow (0.25 mm) at 4.6 % Binder	10.5	8 to 16			
VMA (%) at % 4.6 % Binder	15.7	15.0 (min)			
VFA at 4.6 % Binder	75	65 to 78			

Since all parameters are within acceptance criteria, Optimum Asphalt Content = 4.6 %

Quiz 7



Thank You!!!

