

SCHOOL OF ENGINEERING DEPARTMENT OF CIVIL AND ENVIRONMENTAL ENGINEERING

CEE 3211- MECHANICS OF MATERIALS

DEFLECTION OF BEAMS - Double Integration Method

DEFLECTION OF BEAMS

- Determine the deflection and slope at specific points on beams and shafts, using various analytical methods including:
 - The integration method

APPLICATIONS

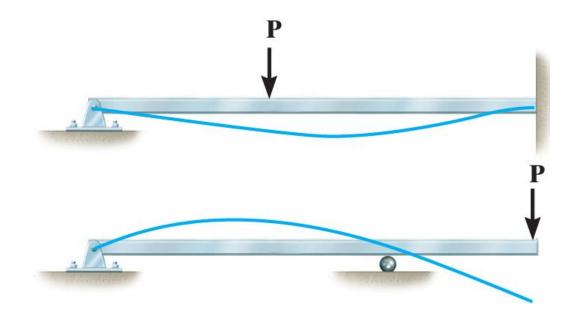






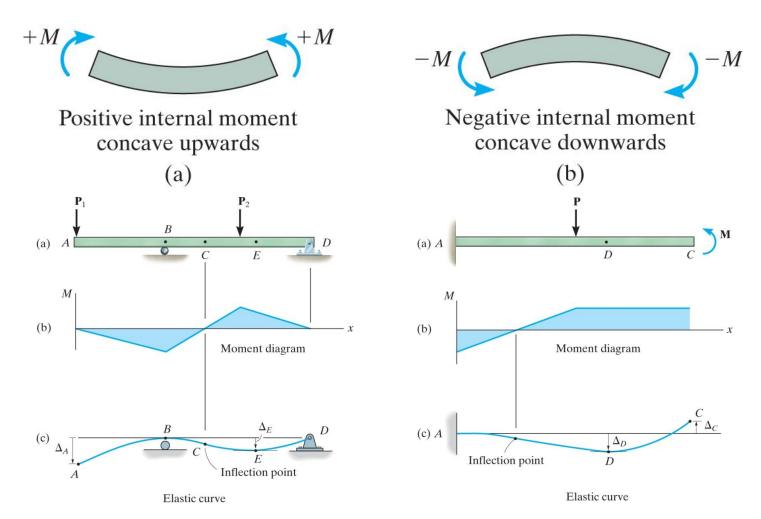
ELASTIC CURVE

 The deflection diagram of the longitudinal axis that passes through the centroid of each cross-sectional area of the beam is called the elastic curve, which is characterized by the deflection and slope along the curve



ELASTIC CURVE (cont)

- Moment-curvature relationship:
 - Sign convention:



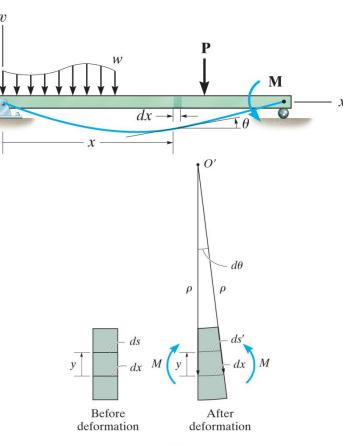
ELASTIC CURVE (cont)

Consider a segment of width dx, the strain in fibre are ds, located at a position y from the neutral axis is ε = (ds' – ds)/ds. However, ds = dx = pdθ and ds' = (p-y) dθ, and so ε = [(p – y) dθ – pdθ] / (pdθ), or

$$\frac{1}{\rho} = -\frac{\varepsilon}{y}$$

Comparing with the Hooke's Law
 ε = σ / E and the flexure formula
 σ = -My/I

$$\frac{1}{\rho} = \frac{M}{EI}$$
 or $\frac{1}{\rho} = -\frac{\sigma}{Ey}$



SLOPE AND DISPLACEMENT BY INTEGRATION

- The equation of the elastic curve is defined by the coordinates v and x. To compute the deflection v = f(x), we must be able to represent the curvature $(1/\rho)$ in terms of v and x.
- Kinematic relationship between radius of curvature ρ and location x:

$$\frac{1}{\rho} = -\frac{d^2 \nu / dx^2}{[1 + (d\nu / dx)^2]^{3/2}}$$

• Then using the moment curvature equation, we have

$$\frac{M}{EI} = \frac{1}{\rho} = \frac{d^2 v/dx^2}{\left[1 + (dv/dx)^2\right]^{3/2}} \approx \frac{d^2 v}{dx^2}$$

SLOPE AND DISPLACEMENT BY INTEGRATION

$$\frac{M}{EI} = \frac{d^2v}{dx^2}$$

- The equation can also be written in two alternative forms
- Differentiate each side with respect to x and substitute V = dM/dx

$$\frac{d}{dx}\left(EI\frac{d^2v}{dx^2}\right) = V(x)$$

• Differentiating again, using w = dV/dx

$$\frac{d^2}{dx^2} \left(EI \frac{d^2 v}{dx^2} \right) = w(x)$$

SLOPE AND DISPLACEMENT BY INTEGRATION

• Flexural rigidity (*EI*) is constant along beam, thus

$$EI\frac{d^4v}{dx^4} = w(x) \text{[LOAD]}$$

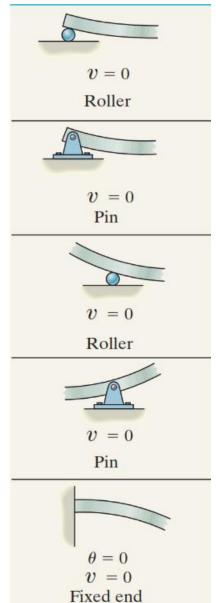
$$EI\frac{d^3v}{dx^3} = V(x)$$
 [Shear]

$$EI\frac{d^2v}{dx^2} = M(x) \text{ [Moment]}$$

• Solution of any of these equations requires successive integrations to obtain *v*.

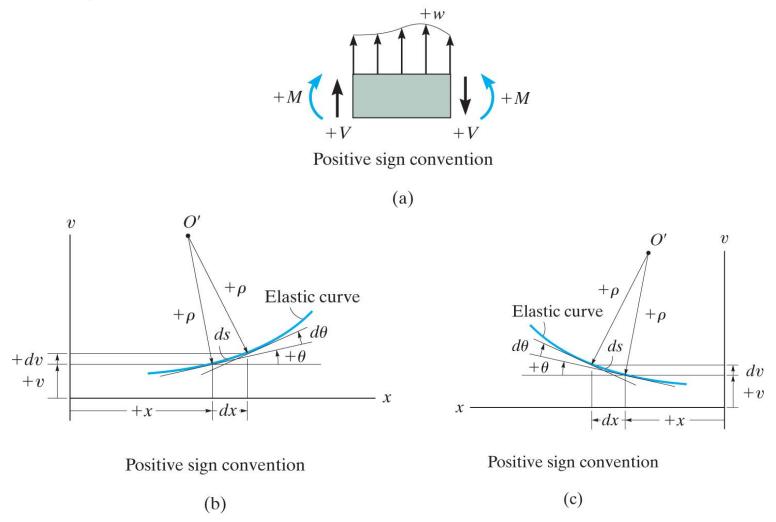
SLOPE AND DISPLACEMENT BY INTEGRATION (cont)

- Boundary Conditions:
 - The integration constants can be determined by imposing the boundary conditions, or
 - Continuity condition at specific locations
- Note, if the beam is supported by a *roller* or *pin*, then it is required that the displacement be *zero* at these points.
- At the fixed support, the *slope* and *displacement* are both *zero*.



SLOPE AND DISPLACEMENT BY INTEGRATION (cont)

• Sign convention:



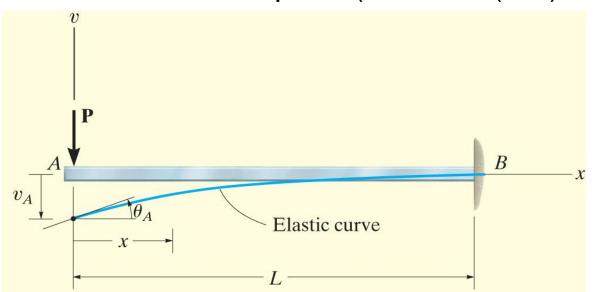
SUMMARY

Deflection =
$$v$$

Slope = $\frac{dv}{dx} = \theta$
Moment $M = EI \frac{d^2 v}{dx^2}$
Shear $V = \frac{dM}{dx} = EI \frac{d^3 v}{dx^3}$ (for *EI* constant)
Load $w = \frac{dV}{dx} = EI \frac{d^4 v}{dx^4}$ (for *EI* constant)

The cantilevered beam shown below is subjected to a vertical load **P** at its end.

- 1. Determine the equation of the elastic curve. *EI* is constant.
- 2. Determine max slope and deflection given the following data: L = 5 m; Load; P = 30 kN) and designed without a factor of safety by assuming the allowable normal stress is equal to the yield stress is 250 MPa; then a W310 x 39 would be found to be adequate ($I = 84.4(10^6)$ mm⁴)



EXAMPLE 1 (cont)

Solutions

- Elastic Curve: shown in the Question figure
- Moment Function: From the free-body diagram, with M acting in the positive direction, we have

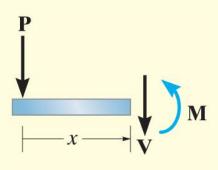
M = -Px

- Slope and Elastic Curve
- Applying $EI\frac{d^2v}{dx^2} = M(x)$ and integrating twice yields

$$EI \frac{d^2 v}{dx^2} = -Px \quad (1)$$

$$EI \frac{dv}{dx} = -\frac{Px^2}{2} + C_1 \quad (2)$$

$$EIv = -\frac{Px^3}{6} + C_1x + C_2 \quad (3)$$



(b)

EXAMPLE 1 (cont)

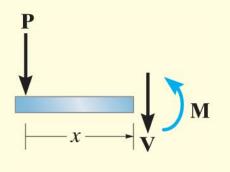
Solutions

• Using the boundary conditions dv/dx = 0 at x = L and v = 0 at x = L, equations 2 and 3 become

$$0 = -\frac{PL^2}{2} + C_1$$

$$0 = -\frac{PL^3}{6} + C_1L + C_2$$

$$\Rightarrow C_1 = \frac{PL^2}{2} \text{ and } C_2 = -\frac{PL^3}{3}$$



(b)

• Substituting these results, with $\theta = dv/dx$, we get

$$\theta = \frac{P}{2EI} \left(L^2 - x^2 \right)$$

Elastic Curve Eqn: $v = \frac{P}{6EI} \left(-x^3 + 3L^2x - 2L^3 \right)$ (Ans)

EXAMPLE 1 (cont)

Solutions

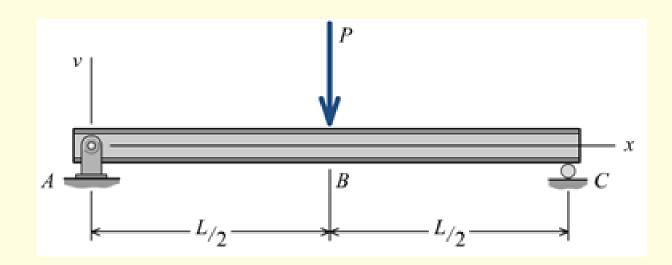
• Maximum slope and displacement occur at for which A(x = 0),

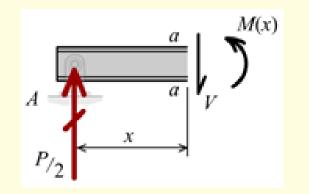
$$\theta_A = \frac{PL^2}{2EI} \quad (4)$$
$$v_A = -\frac{PL^3}{3EI} \quad (5)$$

• Substituting the values of $(L = 5 \text{ m}; \text{Load}; P = 30 \text{ kN}, I = 84.4(10^6) \text{ mm}^4)$

$$\theta_A = \frac{30,000(5,000)^2}{2[200,000][84.4(10^6)]} = 0.0222 \text{ rad}$$
$$v_A = -\frac{30,000(5,000)^3}{3[200,000][84.4(10^6)]} = -74.1 \text{ mm}$$

- For the beam and loading shown below, use the doubleintegration method to determine
- (a) the equation of the elastic curve for segment AB of the beam.
- (b) the deflection at B.
- (c) the slope at A.
- Assume that EI is constant for the beam.. EI is constant.

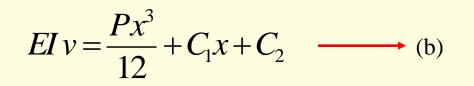




Integration of moment equation:

$$EI\frac{d^2v}{dx^2} = M(x) = \frac{P}{2}x$$
$$= -\frac{dv}{2}Px^2$$

$$EI\frac{dv}{dx} = \frac{Tx}{4} + C_1 \longrightarrow (a)$$



BOUNDARY CONDITIONS

$$v=0$$
 at $x=0$
 $\frac{dv}{dx}=0$ at $x=\frac{L}{2}$

Substitute x = L/2 and dv/dx = 0 into Eq. (a) to determine $C_{1^{12}}$

$$EI(0) = \frac{P(L/2)^2}{4} + C_1 \qquad \qquad \therefore C_1 = -\frac{PL^2}{16}$$

Substitute x = 0 and v = 0 into Eq. (b) to determine C_2 :

$$EI(0) = \frac{P(0)^3}{12} - \frac{PL^2(0)}{16} + C_2 \qquad \therefore C_2 = 0$$

(a) – **ELASTIC CURVE**

$$EIv = \frac{Px^3}{12} - \frac{PL^2x}{16}$$
 $\therefore v = \left[-\frac{Px}{48EI}\left[3L^2 - 4x^2\right]\right] \quad (0 \le x \le \frac{L}{2})$ Ans.

(b) - Deflection @ b

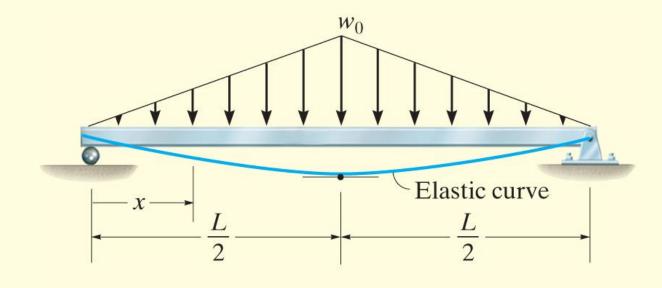
$$v_B = -\frac{P(L/2)}{48EI} \left[3L^2 - 4\left(\frac{L}{2}\right)^2 \right] = \left[-\frac{PL^3}{48EI} \right]$$
 Ans

(b) - **Slope** @ A

$$\frac{dv}{dx}\Big|_{A} = \theta_{A} = \frac{P(0)^{2}}{4EI} - \frac{PL^{2}}{16EI} = \frac{PL^{2}}{16EI}$$

Ans.

The simply supported beam shown below supports the triangular distributed loading. Determine its maximum deflection. *El* is constant.



(a)

EXAMPLE 3 (cont)

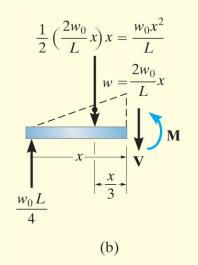
Solutions

• Due to symmetry only one x coordinate is needed for the solution,

 $0 \le x \le L/2$

- The equation for the distributed loading is $w = \frac{2w_0}{L}x$.
- Hence

$$+\sum M_{NA} = 0; \qquad M + \frac{w_0 x^2}{L} \left(\frac{x}{3}\right) - \frac{w_0 L}{4} (x) = 0$$
$$M = -\frac{w_0 x^2}{3L} + \frac{w_0 L}{4} x$$



EXAMPLE 3 (cont)

Solutions

• Integrating twice, we have

$$EI\frac{d^{2}v}{dx^{2}} = M = -\frac{w_{0}}{3L}x^{3} + \frac{w_{0}L}{4}x$$
$$EI\frac{dv}{dx} = -\frac{w_{0}}{12L}x^{4} + \frac{w_{0}L}{8}x^{2} + C_{1}$$
$$EIv = -\frac{w_{0}}{60L}x^{5} + \frac{w_{0}L}{24}x^{3} + C_{1}x + C_{2}$$

• For boundary condition,

•
$$v = 0, x = 0$$
 and

•
$$d\nu/dx = 0, x = L/2$$

$$C_1 = -\frac{5w_0L^3}{192}$$
$$C_2 = 0$$

EXAMPLE 3 (cont)

Solutions

• Hence

$$EIv = -\frac{w_0}{60L}x^5 + \frac{w_0L}{24}x^3 - \frac{5w_0L^3}{192}x$$

• For maximum deflection at x = L/2,

$$v_{\rm max} = -\frac{w_0 L^4}{120 EI} \qquad (Ans)$$